## TRAFFIC AND PARKING IMPACT ASSESSMENT FOR THE PLANNING PROPOSAL FOR A CHANGE IN LAND USE AT LOT 40 DP 1230679 AT GERRINGONG



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## 1 INTRODUCTION

$M^{C}$ Laren Traffic Engineering was commissioned by Allen Price \& Scarratts Pty Ltd (APS) to provide a traffic and parking impact assessment to accompany a planning proposal for a change in land use of Lot 40 DP 1230679 at Gerringong.

### 1.1 Description and Scale of Planning Proposal

The subject planning proposal relates to Lot 40 DP 1230679, Gerringong, which is currently a vacant site on the western side of the Princes Highway, accessible from Sims Road. The area subject consists of approximately 2.95 ha which is currently zoned RU1 - Primary Production and is proposed for general industry and landscape materials supplies development (a concept master plan is presented in Annexure A) which could consist of six (6) proposed industrial buildings. A possible future use of each of these buildings have been provided by Allen Price \& Scarratts Pty Ltd (APS) with the estimated size of each building provided by Edmiston Jones (EJ) and presented in Annexure B. For the purposes of this report, all operations on site are assumed to operate 6-days per week from 6am till 8pm. The assumed future uses of the buildings and surrounding site area are outlined below in Table 1:

TABLE 1: ASSUMED FUTURE USES OF LOT 40

| Building | Possible Use | Scale <br> $($ GFA $)$ | Operation Assumptions |
| :---: | :---: | :---: | :--- |
| A | Wholesale <br> Landscape Supplies | $2,129 \mathrm{~m}^{2}$ | - Wholesale landscape supplies <br> - Yard shed for bulk landscape supplies <br> - Storage for landscaping products for <br> wholesale distribution <br> - Office space and staff amenities |
| B | Firewood Business | $110 \mathrm{~m}^{2}$ | - Yard sales <br> - Yard storage <br> - Office space and staff amenities |
| C | Small Fabrication / <br> Manufacturing <br> Business | $408 \mathrm{~m}^{2}$ | - Product fabrication / storage <br> - Office space and staff amenities |
| D | Small Fabrication / <br> Manufacturing <br> Business | $540 \mathrm{~m}^{2}$ | - Product fabrication / storage <br> - Office space and staff amenities |
| E | Large Fabrication / <br> Manufacturing <br> Business | $1,350 \mathrm{~m}^{2}$ | - Product fabrication / storage <br> - Office space and staff amenities |
| F | Large Fabrication / <br> Manufacturing <br> Business | $1,600 \mathrm{~m}^{2}$ | - Product fabrication / storage <br> - Office space and staff amenities |

Building references are provided within the concept plan provided in Figure 1 below. Note that the proposal is for a change in land use only and the above information is to assist with analysis of the possible future uses of the site. Any subsequent development of the site will be subject to future approval by Kiama Municipal Council.

The concept master plan (Figure 1) outlines that vehicle access to the site will be obtained via two (2) proposed driveways or public road intersections with Sims Road, which include:

- A western connection with Sims Road, approximately 50 m to the north-east of Belinda Street / Sims Road intersection;
- An eastern connection with Sims Road, approximately 250m north-east of the Belinda Street / Sims Road intersection;
- An internal connection between the two proposed driveways / public road intersections.


FIGURE 1: CONCEPT MASTER PLAN FOR THE SITE
The above initial concept master plan above in Figure 1 provides an indicative concept of the proposed driveways / roads and layout of the site.

### 1.2 State Environmental Planning Policy (Infrastructure) 2007

If the planning proposal for change of land use were approved, a subsequent development application for a subdivision and/or buildings would qualify as a traffic generating development with relevant size and/or capacity under Clause 104 of the SEPP (Infrastructure) 2007 as an industrial development of over $5,000 \mathrm{~m}^{2}$ site area, within 90 m of a Classified Road (northbound on-ramp of the Princes Highway). Formal referral to the Transport for NSW (TfNSW) would be necessary. It is also expected that the TfNSW will also be involved in the providing feedback on the planning proposal.

### 1.3 Site Description

The site is currently zoned RU1 - Primary Production, is approximately 2.95 ha in area and is legally identified as Lot 40 DP 1230679. The site is currently vacant land and was most recently used as a site office and depot for the Roads and Martine Service (RMS) during the upgrade of the Princes Highway in this location. The site has multiple high-voltage electrical transmission lines passing through the site as well as gas pipeline easement.

The site has frontage to Sims Road to the east, which runs parallel to the Princes Highway, directly to the north and west of the site are adjacent rural properties zoned RU1 - Primary Production. Sims Road continues to the south of the subject site and joins Belinda Street under the Princes Highway to form an east and west local through traffic corridor below the Princes Highway. The Princes Highway is grade separated and as such no direct access to Princes Highway is permitted within the vicinity of the site. There are on and off ramps serving the Princes Highway that connect with the Sims Road - Belinda Street Road Corridor on either side of the Princes Highway. Further to the east of the site is the township of Gerringong, including Gerringong Train Station located on the opposite side of the Princes Highway.

### 1.4 Site Context

The location of the site based is shown on an aerial photo and a street map in Figure 2 and Figure 3 respectively.


Site Location
FIGURE 2: SITE CONTEXT - AERIAL PHOTO


Site Location
FIGURE 3: SITE CONTEXT - STREET MAP

## 2 EXISTING TRAFFIC AND PARKING CONDITIONS

### 2.1 Road Hierarchy

The road network servicing the site has characteristics as described in the following subsections.

### 2.1.1 Sims Road

- Unclassified Local Road;
- Approximately 6 m wide carriageway facilitating one traffic-flow lane in each direction;
- Default $50 \mathrm{~km} / \mathrm{hr}$ speed limit applies;
- No parking available on either side of the road.


### 2.1.2 Princes Highway

- RMS Classified STATE Road (No. 1);
- Approximately 30 m wide dual carriageway facilitating two traffic-flow lanes in each direction, with 7 m wide on-ramps and off-ramps to Belinda Street in both directions;
- Signposted $100 \mathrm{~km} / \mathrm{h}$ speed limit;
- No parking permitted along either side of the highway.


### 2.1.3 Belinda Street

- RMS Classified REGIONAL Road (No. 571);
- Approximately 11 m wide carriageway facilitating one traffic flow lane in each direction;
- Signposted 50km/hr speed limit;
- Intermittent lengths of kerbside parking available on either side of the road.


### 2.2 Existing Traffic Management

- Give-Way controlled intersection approach from Sims Road to the Sims Road / Belinda Street / Princes Highway (northbound on \& off ramps) intersection;
- STOP controlled intersection approach from the Princes Highway northbound offramp to Sims Road / Belinda Street intersection;
- STOP controlled approach from the Princes Highway southbound off-ramp to Belinda Street intersection.


### 2.3 Existing Traffic Environment

Turning movement count surveys were conducted at the intersections of Sims Road / Belinda Street / Princes Highway (northbound on \& off ramps) and the Belinda Street / Princes Highway (southbound on \& off ramps) from 6:30 AM to 9:30 AM and 2:30 PM to 6:00 PM on Thursday the $11^{\text {th }}$ of February 2021 representing a typical operating weekday.

Additionally, Automatic Traffic Counters (ATCs) traffic tube surveys were undertaken from Thursday the $11^{\text {th }}$ of February 2021 to Thursday the $18^{\text {th }}$ of February 2021 inclusive across both directions of travel of the following roads:

- Sims Road, approximately 260m north of the Sims Road / Belinda Street / Princes Highway (northbound on \& off ramps) intersection.

The locations of the turning movement counts and ATCs on these roads are shown in Annexure C. The full survey results of both the turning movement count surveys and the traffic tube surveys are shown in Annexure D for reference.

### 2.3.1 Intersection Performance

The performance of the surrounding intersections under the existing traffic conditions has been assessed using SIDRA INTERSECTION 9.0. Table 2 summarises the resultant intersection performance data, with full SIDRA results reproduced in Annexure E.

TABLE 2: EXISTING INTERSECTION PERFORMANCES (SIDRA INTERSECTION 9.0)

| Intersection | Peak Hour | Degree of Saturation ${ }^{(1)}$ | Average Delay ${ }^{(2)}$ (sec/veh) | Level of Service ${ }^{(3)}$ | Control Type | Worst Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EXISTING PERFORMANCE |  |  |  |  |  |  |
| Sims Road/ Belinda Street / | AM | 0.14 | $\begin{gathered} 5.3 \\ \text { (Worst: 9.7) } \end{gathered}$ | NA <br> (Worst: A) | Give Way / Stop | T from Princes Highway |
| (northbound on \& off ramps) | PM | 0.20 | $\begin{gathered} \hline 5.8 \\ \text { (Worst: 9.6) } \\ \hline \end{gathered}$ | NA <br> (Worst: A) |  | RT from Princes Highway |
| Belinda Street/ Princes Highway (southbound on \& off ramps) | AM | 0.20 | 3.8 (Worst: 11.8) | NA <br> (Worst: B) | Give Way | RT from Princes Highway |
|  | PM | 0.24 | 4 <br> (Worst: 13.7) | NA <br> (Worst: B) |  | T from Princes Highway |

NOTES:
(1) The Degree of Saturation is the ratio of demand to capacity for the most disadvantaged movement. (2) The average delay is the delay experienced on average by all vehicles. The value in brackets represents the delay to the most disadvantaged movement.
(3) The Level of Service is a qualitative measure of performance describing operational conditions. There are six levels of service, designated from A to F, with A representing the best operational condition and level of service F the worst. The LoS of the intersection is shown in bold, and the LoS of the most disadvantaged movement is shown in brackets. (4) No overall Level of Service is provided for Give Way and Stop controlled intersections as the low delays associated with the dominant movements skew the average delay of the intersection. The Level of Service of the worst approach is an indicator of the operation of the intersection, with a worse Level of Service corresponding to long delays and reduced safety outcomes for that approach.

As shown in Table 2, the intersections of Sims Road / Belinda Street / Princes Highway (northbound on \& off ramps) and the Belinda Street / Princes Highway (southbound on \& off ramps) are currently performing with a high level of efficiency, with a worst movement Level of Service of either "A" or "B" reflected in the AM \& PM peak hour periods in their existing operation.

### 2.3.1 Tube Traffic Survey Results

The results from the tube traffic surveys have been summarised in Table 3.
TABLE 3: TUBE SURVEYS SUMMARY (TWO-WAY VOLUMES)

| Road | Peak Hour Volume |  | Average Weekday <br> Volume | $85^{\text {th }}$Percentile <br> Speed | Heavy <br> Vehicles |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Time | Volume |  |  |  |
| Sims Road | AM <br> $(11 \mathrm{am}-12 \mathrm{am})$ | 16 |  | $66.6 \mathrm{~km} / \mathrm{h}$ | $7.1 \%$ |
|  | PM <br> $(3 \mathrm{pm}-4 \mathrm{pm})$ | 20 | 166 |  |  |

As shown in Table 3 above, the peak hour two-way volumes along Sims Road are 16 vehicles in the AM peak hour of between 11am-12pm and 20 vehicles during the PM peak hour between 3pm4 pm . The $85^{\text {th }}$ Percentile speed at the survey location was $66.6 \mathrm{~km} / \mathrm{h}$, which is higher than the $50 \mathrm{~km} / \mathrm{h}$ signposted speed zoning.

### 2.4 Public Transport and Pedestrian Connectivity

The subject site is within approximately 500 m walking distance of Gerringong Train Station located to the north of the site near Belinda Street, servicing the SCO - South Coast Line. A train service departs every 60-90 minutes in commuter peak periods and provides direct access between Bomaderry and Bondi Junction/Sydney CBD. The location of Gerringong Train Station in relation to our site is identified in Figure 4 below.


FIGURE 4: LOCATION OF GERRINGONG TRAIN STATION
The nearest bus stop from the site is at Gerringong Train Station (ID: 253476) is located approximately 500 m walking distance to the east of the site on Belinda Street. This bus stop services existing bus routes 737 (Bomaderry to Kiama), which operates five times each day in each direction and only makes stops at Berry Train Station, Bomaderry Train Station and Kiama Train Station. This service is operated by Kenbus.

Generally public transport options to the site are extremely limited and are not considered to be a viable or attractive option for staff or customers to the site at the present time. Detailed consideration of footpath connections from the train station to and from the subject site should be considered in future building or subdivision applications for the site.

### 2.5 Future Road and Infrastructure Upgrades

From the TfNSW website and the Kiama Municipal Council's Development Application tracker and website, it appears that there are no future planned road or public transport changes that will affect traffic conditions within the immediate vicinity of the subject site.

## 3 DESIGN CONSIDERATIONS

### 3.1 Road Design Standards

Kiama Municipal Council does not provide any road design requirements for industrial subdivisions or private roads within their development control plan or other similar planning documentation. Internal private or public road design should be based upon the requirements of the maximum design vehicle expected to access the site, such as a 20 m Articulated Vehicle (20m AV) and a 19m Truck \& Dog vehicle. During the detailed design stage of any development application consultation should be undertaken with Council as to the requirements for detailed road design for any proposed private driveways or public roads established as part of the development.

Generally, it is recommended that a minimum internal driveways or public road carriageway widths be such to provide for two parked heavy vehicles on either side of the road $(2.5 \mathrm{~m}$ width each), if necessary, and two heavy vehicles to pass ( 2.5 m each, with 0.5 m clearance on each side and 1 m clearance between moving vehicles), equating to a total road width of 12 m . While this may form the basis of initial road design, the recommended road width may change during the detailed design stage. If parking is not proposed along either side of the internal proposed private or public roads, the road carriageway width may be further reduced. It is noted that the 12 m width accounts only for the road carriageway and that additional width will be required to accommodate kerb, verge and footpath provisions.

### 3.2 NSW RFS - "Planning for Bushfire Protection"

The site has not been identified by the NSW Rural Fire Service as bushfire prone land and as such is not required to meet the objectives of the NSW RFS Planning for Bushfire Protection 2019.

### 3.3 Proposed Intersection/Road Access Points

The provided concept plan includes two vehicular access points to Sims Road. It is expected based on the proposed nature and scale of the concept plan that both road access points would be required to serve the development, particularly considering that vehicles up to 20 m long will be facilitated and would otherwise be required to turn around within the site.

The detailed design of these points of access will need to be further considered as part of any Development Application to improve the site.

### 3.4 Heavy Vehicle Access

It has been advised by APS that the largest proposed vehicle to use any of the proposed land uses on the site is a 20 m Articulated Vehicle (AV) or a 19 m Truck and Dog. Given the location of the proposed site with respect to the Princes Highway on and off ramps, the site is considered optimal for use by heavy vehicles having immediate access to the highway without impact on any surrounding residential streets. The on and off-ramps and their associated intersections have been designed to accommodate vehicles up to the proposed maximum size being a 20 m AV. Swept path testing has been undertaken of both a 20 m AV and 19m Truck and Dog at these intersections, proving successful, with the resultant swept paths being provided in Annexure F for reference.

The internal road design of the site shall be designed in future development applications to accommodate the maximum design vehicle for the site, along with any internal proposed driveways which should comply with relevant driveway requirements of AS2890.1:2004 and AS2890.2:2018.

## 4 PARKING ASSESSMENT

### 4.1 DCP Car Parking Requirement

Reference is made to Kiama Municipal Council's Development Control Plan 2020 Chapter 3 - Common Requirements which designates the following range of parking requirements applicable to the masterplan:
Industrial
Light industry (if not otherwise defined below)
Whichever is the greater -2 spaces per unit of 1.3 spaces per
$100 \mathrm{~m}^{2}$ gross floor area
Warehouse or distribution centres
1 space per $300 m^{2}$ gross floor area
Parking requirements will be determined by Council following the
completion and submission of a Parking Impact Study by a
suitably qualified and experienced professional person

Bulky goods premises
1 space per $50 m^{2}$ gross floor area.
Self storage units
Whichever is the greater - 4 space or 1 space per 50 storage units


#### Abstract

3.6.4. Calculations should be rounded up to the nearest whole number for each use on the site and then combined to give the total amount. For example, if the calculation determines for residential component that 5.3 spaces are required and for commercial component 8.7 spaces are required than 15 spaces would be required in total.


As a conservative estimate for the sites parking requirements, the Light Industrial car parking requirements as provided in the DCP has been adopted broadly to all proposed buildings within the site. It is noted that when the exact uses of each building have been finalised, during the development application stage, the overarching parking requirement can be further refined based on the known operational details of the development. The estimated car parking requirements for the site are summarised in Table 4 below.

TABLE 4: COUNCIL DCP INDUSTRIAL CAR PARKING REQUIREMENTS

| Building | Type | Scale <br> (GFA) | Rate | Parking Provision |
| :---: | :---: | :---: | :---: | :---: |
| A | Industrial | 2,129m² | 1.3 spaces per $100 \mathrm{~m}^{2}$ | 28 |
| B |  | $110 \mathrm{~m}^{2}$ |  | 12 |
| C |  | $408 \mathrm{~m}^{2}$ |  | 6 |
| D |  | $540 \mathrm{~m}^{2}$ |  | 8 |
| E |  | 1,350m ${ }^{2}$ |  | 18 |
| F |  | 1,600m ${ }^{2}$ |  | 21 |
|  |  |  |  | 93 |

As identified above, assuming a worst-case (in terms of likely parking requirements), the site would require a combined total of 93 parking spaces across the development. As noted earlier, at the development application stage, where detailed land-use and operational characteristics are known for each building, parking requirements can be refined and likely reduced. Nonetheless, there is sufficient room on the site for enough parking to be provided, to meet the requirements of the Council DCP.

It is noted that the intended use of Building A has been more refined than other the five buildings at this stage with the intended use being a Wholesale Landscape Supplies business. As such this land use is likely to require a lower parking provision than identified above, as such the Kiama DCP Warehouse rate may be more applicable to the building. Application of the Warehouse parking rate of " 1 space per 300m" to Building A, results in a total parking provision of 8 spaces.

The exact parking provision for each building can be clearly determined at the development application stage for the site and is likely to vary from what is outlined above. There is sufficient room on the site for the required parking provision to be fully accommodated.

### 4.2 Disabled Parking

Reference is made to Kiama Municipal Council's Development Control Plan 2020 Chapter 3 - Common Requirements which outlines the following with regard to the provision of disable parking:
3.5.13 To ensure an adequate parking provision for people with disability, the minimum requirements for Class 3,5,6, 7, 8,9 buildings as defined in the Building Code of Australia (BCA) shall be included within the development.

The BCA provides the following requirements for Class 5, 7, 8 or 9c which is most likely to apply to the proposed developments:

1 space for every 100 carparking spaces or part thereof.
There is sufficient room on-site for the provision of the appropriate number of disabled spaces. If the site is subdivided and each building is operated independently (i.e., separate parking areas are provided for each building) each individual in isolation shall provide the appropriate number of disabled spaces in accordance with the BCA requirements.

### 4.3 Bicycle and Motorcycle Parking

Reference is made to Kiama Municipal Council's Development Control Plan 2020 Chapter 3 - Common Requirements which outlines the following with regard to bicycle parking provision:
> 3.6.45 Provision of bicycle parking shall be made in accordance with the 'Cycling Aspects of Austroads Guide’ (2011) as per the extract in Note C. Where it can be demonstrated that there is sufficient under-utilised bicycle parking in the vicinity of the proposal of that bicycle parking is not warranted in the circumstances, Council may totally or partially waiver this requirement.

Further reference is made to Appendix 5 Bicycle Parking Requirements of Chapter 3 of the DCP, which provides an extract of the Austroads, outlining the following bicycle parking provision for industrial uses:

General Industry

$$
1 \text { per 150m² GFA (Employee Parking Spaces, Class } 1 \text { or 2) }
$$

## Light Industry

## 1 per 1000m² GFA (Employee Parking spaces, Class 1 or 2)

Depending on the final determination of the exact uses for each building, the above bicycle parking rates are likely to apply. Given the location of the site, bicycle usage is expected to be minimal, and as such this bicycle provision is likely to be reduced through consultation with Council. Regardless, there is ample room within the proposed site to provide for the resultant bicycle parking requirements.

The Kiama DCP does not stipulate any requirement for motorcycle parking for an industrial development.

### 4.4 Servicing \& Loading

Reference is made to Kiama Municipal Council's Development Control Plan 2020 Chapter 3 - Common Requirements which outlines the following with regard to servicing and loading requirements:
3.6.35 To ensure that adequate space is provided for manoeuvring of vehicles, turning paths and heights for vehicle access and parking shall be based upon the largest vehicle likely to utilise the premises, as defined by AS2890. At a minimum these are:

- Commercial Zoned Development (sites <600 m²) - Small Rigid Vehicle (SRV)
- Commercial Zoned Development (sites 600+ m²) - Medium Rigid Vehicle (MRV)
- Industrial Zoned Development - Heavy Rigid Vehicle
- All sites - size of garbage collection vehicle to service the site.

The internal private road / driveway or public roads and loading areas of the subject site should be designed to cater for the sites largest design vehicle, being a 20 m AV or 19 m Truck and Dog. Each individual building that will be created will need to have sufficient room to cater for the provision the appropriate service vehicles and waste collection in accordance with both the DCP and operational needs of each building. The detailed design of each individual building, with regard to access and manoeuvring of service vehicles, will be undertaken at the development application stage of the project.

### 4.5 Car Park and Loading Facilities Design \& Compliance

If the planning proposal were to be approved, the driveway access and on-site parking areas for each building would be required to comply with the relevant clauses and objectives of AS2890.1:2004, AS2890.2:2018, AS2890.6.2009. Any proposed subdivision of the site would generally be able to accommodate suitable driveway locations and suitable car parking provision on each individual building. If approved, a detailed assessment of these requirements is expected to be undertaken during detailed DA stage.

## 5 TRAFFIC ASSESSMENT

The impact of the expected traffic generation levels associated with the subject proposal is discussed in the following sub-sections.

### 5.1 Traffic Generation

Reference is made to the TfNSW (Previously RMS) Guide to Traffic Generating Developments and more recent technical directions including TDT2013/04a. The Guide and its supplements outline the below traffic generation rates with respect to industrial and commercial land uses:

Business parks and industrial estate - Regional
AM Peak (1 hour)
0.70 vehicle trips per 100m² GFA (Range: 0.32-1.20)

PM Peak (1 hour)
0.78 vehicle trips per $100 m^{2}$ GFA (Range: $0.39-1.30$ )

It should be noted that these TfNSW traffic generation rates include the traffic generation associated with all vehicles associated with the site including visitors, staff and commercial vehicles.

As the exact operations and uses across the site may vary between the submission of the planning proposal and the submission of an individual development application, a general conservative approach has been adopted in terms of traffic generation for the site.

Based on the above TfNSW rates, the estimated peak hour traffic generation has been estimated using both the average generation rate and ( 0.70 vehicle trips per $100 \mathrm{~m}^{2}$ GFA in the AM and 0.78 vehicle trips per $100 \mathrm{~m}^{2}$ GFA in the PM) the high range generation rates ( 1.20 vehicle trips per $100 m^{2}$ GFA in the AM and 1.30 vehicle trips per $100 m^{2}$ GFA in the PM ) for industrial estates, with the resultant traffic generation outlined in Table 5 and Table 6 below.

TABLE 5: AVERAGE PEAK HOUR TRAFFIC GENERATION (TFNSW GUIDE)

| Building | Type | Scale (GFA) | Rate <br> (trips per 100m² GFA) |  | Generated Trips |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | AM | PM | AM | PM |
| A | Industrial | 2,129m² | 0.70 | 0.78 | $\begin{gathered} 15 \\ (12 \mathrm{in}, 3 \text { out }) \end{gathered}$ | $\begin{gathered} 17 \\ (3 \mathrm{in}, 14 \text { out }) \end{gathered}$ |
| B |  | $110 \mathrm{~m}^{2}$ |  |  | $\begin{gathered} 1 \\ (1 \mathrm{in}, 0 \text { out }) \end{gathered}$ | $\begin{gathered} 1 \\ (0 \text { in, } 1 \text { out }) \end{gathered}$ |
| C |  | 408m ${ }^{2}$ |  |  | $\begin{gathered} 3 \\ (2 \text { in, } 1 \text { out }) \end{gathered}$ | $\begin{gathered} 3 \\ (1 \mathrm{in}, 2 \text { out }) \end{gathered}$ |
| D |  | $540 \mathrm{~m}^{2}$ |  |  | $\begin{gathered} 4 \\ (3 \text { in, } 1 \text { out }) \end{gathered}$ | $\begin{gathered} 4 \\ (1 \mathrm{in}, 3 \text { out }) \\ \hline \end{gathered}$ |
| E |  | 1,350m² |  |  | $\begin{gathered} 9 \\ (7 \mathrm{in}, 2 \text { out }) \end{gathered}$ | $\begin{gathered} 11 \\ (2 \text { in, } 9 \text { out }) \end{gathered}$ |
| F |  | 1,600m ${ }^{2}$ |  |  | $\begin{gathered} 11 \\ (9 \mathrm{in}, 2 \text { out }) \end{gathered}$ | $\begin{gathered} 12 \\ (2 \mathrm{in}, 10 \text { out }) \end{gathered}$ |
| Total |  | 6,137m ${ }^{2}$ |  |  | $\begin{gathered} 43 \\ (34 \text { in, } 9 \text { out }) \end{gathered}$ | $\begin{gathered} 48 \\ (9 \mathrm{in}, 39 \text { out }) \end{gathered}$ |

## Notes:

(1) In the AM peak hour $80 \%$ of trips are inbound and $20 \%$ of trips are outbound, and vice versa in the PM peak hour.

As shown above, it is expected that the proposed development based on the concept master plan in the average case will generate approximately 43 ( $34 \mathrm{in}, 9$ out) and 48 ( 9 in, 39 out) vehicle trips during the AM and PM peak hours, respectively.

TABLE 6: HIGH RANGE PEAK HOUR TRAFFIC GENERATION (TFNSW GUIDE)

| Building | Type | Scale <br> (GFA) | Rate <br> (trips per 100m² GFA) |  | Generated Trips |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | AM | PM | AM | PM |
| A | Industrial | 2,129m² | 1.2 | 1.3 | $\begin{gathered} 26 \\ (21 \text { in, } 5 \text { out }) \end{gathered}$ | $\begin{gathered} 28 \\ (6 \text { in, } 22 \text { out }) \end{gathered}$ |
| B |  | $110 \mathrm{~m}^{2}$ |  |  | $\begin{gathered} 1 \\ (1 \mathrm{in}, 0 \text { out }) \end{gathered}$ | $\begin{gathered} 1 \\ (0 \text { in, } 1 \text { out }) \end{gathered}$ |
| C |  | 408m ${ }^{2}$ |  |  | $\begin{gathered} 5 \\ (4 \mathrm{in}, 1 \text { out }) \end{gathered}$ | $\begin{gathered} 5 \\ (1 \mathrm{in}, 4 \text { out }) \end{gathered}$ |
| D |  | $540 \mathrm{~m}^{2}$ |  |  | $\begin{gathered} 6 \\ (5 \mathrm{in}, 1 \text { out }) \end{gathered}$ | $\begin{gathered} 7 \\ (1 \mathrm{in}, 6 \text { out }) \end{gathered}$ |
| E |  | 1,350m ${ }^{2}$ |  |  | $\begin{gathered} 16 \\ \text { (13 in, } 3 \text { out) } \end{gathered}$ | $\begin{gathered} 18 \\ (4 \mathrm{in}, 14 \text { out }) \end{gathered}$ |
| F |  | 1,600m ${ }^{2}$ |  |  | $\begin{gathered} 19 \\ \text { (15 in, } 4 \text { out) } \end{gathered}$ | $\begin{gathered} 21 \\ (4 \text { in, } 17 \text { out }) \end{gathered}$ |
| Total |  | 6,137m ${ }^{2}$ |  |  | $\begin{gathered} 73 \\ \text { (59 in, } 14 \text { out) } \end{gathered}$ | $\begin{gathered} 80 \\ \text { (16 in, } 64 \text { out) } \end{gathered}$ |

Notes:
(1) In the AM peak hour $80 \%$ of trips are inbound and $20 \%$ of trips are outbound, and vice versa in the PM peak hour.

As shown above, it is expected that the high range traffic generation of the site based on the concept master plan (TfNSW Guide maximum rates of regional areas) will be approximately 73 (59 in, 14 out) and 80 ( $16 \mathrm{In}, 64$ Out) vehicle trips during the AM and PM peak hours, respectively.
The above traffic generation rates include staff, visitors, and commercial vehicles and as such provide an estimated total generation for the site at both the average and the high range.

### 5.2 Traffic Assignment

The road network and the locations of residential areas and towns surrounding the site have been assessed and the following traffic assignment has been assumed for all traffic to and from the site:

- $45 \%$ to/from the North via the Princes Highway;
- $45 \%$ to/from the South via the Princes Highway;
- $10 \%$ to/from the East via Belinda Street;


### 5.3 Traffic Impact

The traffic generation and distribution outlined in Section 5.1 and Section 5.2 above has been added to the existing traffic volumes estimated in Section 2.3. SIDRA INTERSECTION 9.0 was used to assess the intersections performance of these intersections both under the "average" traffic generation case (Table 5) and the "high range" traffic generation case (Table 6)

The performance of each intersection with the impact of the planning proposal's indicative traffic generation has been summarised in Table 7 below.

TABLE 7: FUTURE INTERSECTION PERFORMANCES

| Intersection | Peak Hour | Degree of Saturation ${ }^{(1)}$ | Average Delay ${ }^{(2)}$ (sec/veh) | Level of Service ${ }^{(3)}$ | Control Type | Worst Movement |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EXISTING PERFORMANCE |  |  |  |  |  |  |
| Sims Road Belinda Street / Princes Highway (northbound on \& off ramps) | AM | 0.14 | $\begin{gathered} \hline 5.3 \\ \text { (Worst: 9.7) } \end{gathered}$ | NA (Worst: A) | Give Way / Stop | T from Princes Highway |
|  | PM | 0.20 | 5.8 (Worst: 9.6) | NA (Worst: A) |  | RT from Princes Highway |
| Belinda Street / Princes Highway (southbound on \& off ramps) | AM | 0.20 | $\begin{gathered} 3.8 \\ \text { (Worst: 11.8) } \end{gathered}$ | $\begin{gathered} \hline \text { NA } \\ \text { (Worst: B) } \end{gathered}$ | Give Way | RT from Princes Highway |
|  | PM | 0.24 | 4 (Worst: 13.7) | NA (Worst: B) |  | T from Princes Highway |
| FUTURE PERFORMANCE - TfNSW Average Traffic Generation |  |  |  |  |  |  |
| Sims Road / Belinda Street / Princes Highway (northbound on \& off ramps) | AM | 0.16 | $\begin{gathered} 5.2 \\ \text { (Worst: 9.9) } \end{gathered}$ | $\begin{gathered} \hline \text { NA } \\ \text { (Worst: A) } \end{gathered}$ | Give Way / Stop | T from Princes Highway |
|  | PM | 0.22 | 6 (Worst: 9.7) | $\begin{gathered} \text { NA } \\ \text { (Worst: A) } \end{gathered}$ |  | RT from Princes Highway |
| Belinda Street / Princes Highway (southbound on \& off ramps) | AM | 0.21 | (Worst: 11.1) | $\begin{gathered} \hline \text { NA } \\ \text { (Worst: B) } \end{gathered}$ | Give Way | T from Princes Highway |
|  | PM | 0.26 | $\begin{gathered} 4.2 \\ \text { (Worst: 14.6) } \end{gathered}$ | $\begin{gathered} \hline \text { NA } \\ \text { (Worst: B) } \end{gathered}$ |  | T from Princes Highway |
| FUTURE PERFORMANCE - TfNSW High Traffic Generation |  |  |  |  |  |  |
| Sims Road / Belinda Street / Princes Highway (northbound on \& off ramps) | AM | 0.17 | $\begin{gathered} 5.2 \\ \text { (Worst: 10) } \end{gathered}$ | NA (Worst: B) | Give Way / Stop | T from Princes Highway |
|  | PM | 0.23 | 6 (Worst: 9.9) | $\begin{gathered} \hline \text { NA } \\ \text { (Worst: A) } \end{gathered}$ |  | RT from Princes Highway |
| Belinda Street / Princes Highway (southbound on \& off ramps) | AM | 0.21 | $\begin{gathered} 4.1 \\ \text { (Worst: 11.2) } \\ \hline \end{gathered}$ | NA (Worst: B) | Give Way | T from Princes Highway |
|  | PM | 0.27 | $\begin{gathered} 4.2 \\ \text { (Worst: 14.9) } \end{gathered}$ | $\begin{gathered} \hline \text { NA } \\ \text { (Worst: B) } \end{gathered}$ |  | T from Princes Highway |

Note: For Table 7, please also refer to the notes also provided in association with Table 2. Scenario 1 has been analysed using SIDRA Intersection 9.0.

As can be seen from the above SIDRA Intersection results all both the intersection of Sims Road / Belinda Street / Princes Highway (northbound on \& off ramps) and Belinda Street / Princes Highway (southbound on \& off ramps) retain their current worst movement Level of Service (LoS) of "A" or "B" under future conditions with the impacts of the planning proposal. These results indicate minimal delays and additional capacity, indicating that there will be negligible impact on the existing road network as a result of the planning proposal.

It should be noted that Give Way and Stop controlled intersections, in some circumstances, simply examining the highest individual average delay can be misleading. The size of the movement with the highest average delay per vehicle should also be taken into account. Thus, for example, an intersection where all movements are operating at a level of Service A, except one which is at level of Service E, may not necessarily define the intersection level of service as E if that movement is very small. That is, longer delays to a small number of vehicles may not justify upgrading an intersection unless a safety issue was also involved.

## 6 CONCLUSION

The traffic, road design, road safety and parking impacts of the subject Planning Proposal for a Change in Land Use at Lot 40 DP 1230679 at Gerringong, as shown in the plans provided in Annexure A to this report, have been assessed. With the following items important to note:

- The proposal will only result in the change of land use of the land;
- Application of Council's DCP rates for Light Industrial (as a worst-case parking scenario), requires the provision of 93 car parking spaces across the site for the current concept masterplan. When the exact operational details of each individual building are known a more refined parking requirement can be determined during the development application stage.
- If the site is subdivided after the change of land use and each building is operated independently (i.e., separate parking areas are provided for each building) each individual building in isolation shall provide the appropriate number of disabled spaces in accordance with the BCA requirements.
- The detailed design of the internal road network (either public or private) shall be assessed at the DA Stage but shall be designed such as to accommodate the largest commercial vehicle intended to use the site, which is currently identified as a 20 m AV and a 19m Truck and Dog.
- The internal car park and loading facilities within each building is to be designed in accordance with AS2890.1:2004, AS2890.2.2018 and AS2890.6:2006. The detailed design of the internal car parks and loading facilities will be reviewed during the development application stage.
- The immediate external road network (Sims Road, Belinda Street and the Princes Highway on and off ramps) have been assessed with Swept Path Testing of a 20m AV and 19m Truck and Dog and have sufficient width and intersection design to accommodate these large vehicles accessing the site. The results have been reproduced within Annexure F below.
- The traffic generated by the proposed change of land use has been found to range between 43 ( $34 \mathrm{in}, 9$ out) and 73 ( $59 \mathrm{in}, 14$ out) vehicle trips during the AM peak and 48 ( $9 \mathrm{in} ; 39$ out) and 80 ( $16 \mathrm{in}, 64$ out) during the PM peak hour, under the average and high-range case, respectively. SIDRA Intersection analysis indicates that the traffic generation of the proposal in both the average and high range cases, would have a negligible impact on the existing road network surrounding the site. There are no changes to the existing Level of Service of the surrounding intersections due the future traffic generation of the planning proposal.

In view of the foregoing, the subject planning proposal is supportable in terms of traffic flow, road safety and parking impacts. It is recognised that this assessment has been informed by a concept masterplan for the site and a more detailed assessment would be required when considering the future buildings or subdivision.

## ANNEXURE A: CONCEPT MASTERPLAN

(1 SHEET)


## ANNEXURE B: SCHEDULE OF AREAS (GFA)

BASED ON THE CONCEPT MASTERPLAN
(1 SHEET)

Project: Visual Impact study - Gerringong
For: Allen Price \& Scarratts
Job No: 20-0064
Date: 11/2/21-1

## SCHEDULE OF AREAS

| BUILDING | RIDGE <br> HEIGHT <br> (m) | EXTERNAL <br> WALL <br> HEIGHT <br> (m) | MAX <br> LENGTH <br> (m) | MAX <br> SPAN <br> (m) | GROUND <br> FLOOR <br> AREA <br> (m2) | FIRST <br> FLOOR <br> AREA <br> (m2) | TOTAL <br> FLOOR <br> AREA <br> (m2) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 11 | 8.5 | 87.5 | 25 | 2029 | 100 | 2129 |
| B | 9 | 7.1 | 12 | 12 | 110 |  | 110 |
| C | 11 | 7.4 | 34 | 12 | 408 |  | 408 |
| D | 11 | 9.1 | 45 | 12 | 540 |  | 540 |
| E | 11 | 7.4 | 50 | 25 | 1250 | 100 | 1350 |
| F | 11 | 7.4 | 60 | 25 | 1500 | 100 | 1600 |
| TOTAL FLOOR AREA |  |  |  |  | 5837 | 300 | 6137 |

ANNEXURE C: TRAFFIC COUNT LOCATIONS
(1 SHEET)

Site Snapshot and Survey Location

$\square$ Site
Intersection
7-Day Tube Count

## ANNEXURE D: TRAFFIC SURVEY RESULTS

(SHEET 1 OF 3)

TRANS TRAFFIC SURVEY $=\Longrightarrow$
TURNING MOVEMENT SURVEY
Intersection of On-ramp Princess Hwy and Sims Rd, Gerringong

| GPS | $-34.744854,150.815167$ |
| :--- | :--- |
| Date: | Thu 11/02/21 |
| Weather: | Overcast |
| Suburban: | Gerringong |
| Customer: | McLaren |



| Survey | AM: | 6:30 AM-9:30 AM |
| :---: | :---: | :--- |
| Period | PM: | $2: 30$ PM-6:00 PM |
| Traffic | AM: | $8: 15$ AM-9:15 AM |
|  | Peak | PM: |
| Pa: | $3: 15$ PM-4:15 PM |  |


| All Vehicles |
| :---: |
| Time |


| Time |  | North Approach Sims Rd |  |  |  | East Approach On-ramp Princess Hw |  |  |  | South Approach Belinda St |  |  |  | Vest Approach Off-ramp Princess Hw |  |  |  | Hourly Total |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Period Start | Period End | U | R | SB | L | U | R | WB | L | U | R | NB | L | U | R | EB | L | Hour | Peak |
| 6:30 | 6:45 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 28 | 2 | 2 | 0 | 6 | 0 | 0 | 226 |  |
| 6:45 | 7:00 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 32 | 2 | 6 | 0 | 25 | 0 | 0 | 251 |  |
| 7:00 | 7:15 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 30 | 3 | 3 | 0 | 22 | 2 | 0 | 263 |  |
| 7:15 | 7:30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 39 | 0 | 2 | 0 | 17 | 0 | 1 | 278 |  |
| 7:30 | 7:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 49 | 0 | 2 | 0 | 13 | 0 | 0 | 305 |  |
| 7:45 | 8:00 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 52 | 2 | 2 | 0 | 17 | 2 | 0 | 318 |  |
| 8:00 | 8:15 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 52 | 1 | 2 | 0 | 20 | 1 | 0 | 345 |  |
| 8:15 | 8:30 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 58 | 4 | 3 | 0 | 18 | 2 | 0 | 357 | Peak |
| 8:30 | 8:45 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 49 | 2 | 3 | 0 | 18 | 2 | 0 | 348 |  |
| 8:45 | 9:00 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 64 | 5 | 5 | 0 | 28 | 1 | 0 |  |  |
| 9:00 | 9:15 | 0 | 0 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 52 | 4 | 5 | 0 | 19 | 2 | 1 |  |  |
| 9:15 | 9:30 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 45 | 0 | 3 | 0 | 25 | 0 | 2 |  |  |
| 14:30 | 14:45 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 56 | 0 | 1 | 0 | 20 | 1 | 0 | 385 |  |
| 14:45 | 15:00 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 55 | 3 | 0 | 0 | 32 | 1 | 0 | 418 |  |
| 15:00 | 15:15 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 59 | 2 | 3 | 0 | 26 | 2 | 1 | 427 |  |
| 15:15 | 15:30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 61 | 0 | 5 | 0 | 49 | 3 | 0 | 434 | Peak |
| 15:30 | 15:45 | 0 | 0 | 4 | 2 | 0 | 0 | 0 | 0 | 0 | 56 | 8 | 5 | 0 | 34 | 3 | 0 | 408 |  |
| 15:45 | 16:00 | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 0 | 1 | 52 | 3 | 3 | 0 | 41 | 0 | 0 | 417 |  |
| 16:00 | 16:15 | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 60 | 1 | 2 | 0 | 32 | 3 | 0 | 411 |  |
| 16:15 | 16:30 | 0 | 0 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 52 | 5 | 2 | 0 | 26 | 4 | 0 | 407 |  |
| 16:30 | 16:45 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 76 | 0 | 4 | 0 | 38 | 1 | 0 | 417 |  |
| 16:45 | 17:00 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 47 | 2 | 6 | 0 | 34 | 4 | 0 | 368 |  |
| 17:00 | 17:15 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 62 | 1 | 1 | 0 | 31 | 1 | 0 | 325 |  |
| 17:15 | 17:30 | 0 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 44 | 7 | 5 | 0 | 40 | 3 | 0 |  |  |
| 17:30 | 17:45 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 39 | 2 | 0 | 0 | 29 | 0 | 0 |  |  |
| 17:45 | 18:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 24 | 1 | 6 | 0 | 20 | 2 | 1 |  |  |


| Peak Time |  | North Approach Sims Rd |  |  |  | East Approach On-ramp Princess Hw |  |  |  | South Approach Belinda St |  |  |  | Nest Approach Off-ramp Princess Hw |  |  |  | Peak total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Period Start | Period End | U | R | SB | L | U | R | WB | L | U | R | NB | L | U | R | EB | L |  |
| 8:15 | 9:15 | 0 | 0 | 7 | 5 | 0 | 0 | 0 | 0 | 0 | 223 | 15 | 16 | 0 | 83 | 7 | 1 | 357 |
| 15:15 | 16:15 | 0 | 0 | 8 | 4 | 0 | 0 | 0 | 0 | 1 | 229 | 12 | 15 | 0 | 156 | 9 | 0 | 434 |

$\begin{aligned} & \text { Note: Site sketch is for illustrating traffic flows. Direction is indicative only, drawing is not to scale and not an exact streets configuration. } \\ & \text { Graphic } \\ & \text { Total } \\ & \text { Light } \\ & \text { Heavy }\end{aligned}$

## ANNEXURE D: TRAFFIC SURVEY RESULTS

(SHEET 2 OF 3)

TRANS TRAFFIC SURVEY \# $\ldots$
TURNING MOVEMENT SURVEY
Intersection of Off-ramp Princess Hwy and Belinda St, Gerringong


|  | AM: | 6:30 AM-9:30 AM |
| :---: | :---: | :---: |
| Period | PM: | 2:30 PM-6:00 PM |
| Traffic | AM: | 7:30 AM-8:30 AM |
| Peak |  | 3:45 PM-4:45 PI |


| Time |  | North Approach Belinda St |  |  |  | East Approach Off-ramp Princess Hw |  |  |  | South Approach Belinda St |  |  |  | Vest Approach On-ramp Princess Hw |  |  |  | Hourly Total |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Period Start | Period End | 0 | R | SB | L | U | R | WB | L | $u$ | R | NB | L | $\checkmark$ | R | EB | L | Hour | Peak |
| 6:30 | 6:45 | 0 | 0 | 6 | 0 | 0 | 2 | 0 | 25 | 0 | 0 | 30 | 28 | 0 | 0 | 0 | 0 | 491 |  |
| 6:45 | 7:00 | 0 | 1 | 25 | 0 | 0 | 4 | 1 | 57 | 0 | 0 | 36 | 37 | 0 | 0 | 0 | 0 | 540 |  |
| 7:00 | 7:15 | 0 | 0 | 24 | 0 | 0 | 1 | 0 | 30 | 0 | 0 | 35 | 35 | 0 | 0 | 0 | 0 | 545 |  |
| 7:15 | 7:30 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 32 | 0 | 0 | 41 | 24 | 0 | 0 | 0 | 0 | 563 |  |
| 7:30 | 7:45 | 0 | 1 | 12 | 0 | 0 | 1 | 0 | 36 | 0 | 0 | 50 | 40 | 0 | 0 | 0 | 0 | 588 | Peak |
| 7:45 | 8:00 | 0 | 0 | 18 | 0 | 0 | 2 | 0 | 51 | 0 | 0 | 54 | 41 | 0 | 0 | 0 | 0 | 578 |  |
| 8:00 | 8:15 | 0 | 1 | 20 | 0 | 0 | 0 | 0 | 36 | 0 | 0 | 55 | 31 | 0 | 0 | 0 | 0 | 571 |  |
| 8:15 | 8:30 | 0 | 1 | 18 | 0 | 0 | 0 | 0 | 31 | 0 | 0 | 65 | 24 | 0 | 0 | 0 | 0 | 566 |  |
| 8:30 | 8:45 | 0 | 0 | 19 | 0 | 0 | 2 | 0 | 37 | 0 | 0 | 52 | 20 | 0 | 0 | 0 | 0 | 563 |  |
| 8:45 | 9:00 | 0 | 0 | 29 | 0 | 0 | 4 | 0 | 31 | 0 | 0 | 70 | 25 | 0 | 0 | 0 | 0 |  |  |
| 9:00 | 9:15 | 0 | 4 | 19 | 0 | 0 | 3 | 0 | 31 | 0 | 0 | 58 | 23 | 0 | 0 | 0 | 0 |  |  |
| 9:15 | 9:30 | 0 | 1 | 25 | 0 | 0 | 1 | 0 | 34 | 0 | 0 | 47 | 28 | 0 | 0 | 0 | 0 |  |  |
| 14:30 | 14:45 | 0 | 1 | 19 | 0 | 0 | 1 | 0 | 59 | 0 | 0 | 56 | 22 | 0 | 0 | 0 | 0 | 678 |  |
| 14:45 | 15:00 | 0 | 1 | 31 | 0 | 0 | 1 | 0 | 50 | 0 | 0 | 57 | 28 | 0 | 0 | 0 | 0 | 727 |  |
| 15:00 | 15:15 | 0 | 4 | 22 | 0 | 0 | 3 | 0 | 54 | 0 | 0 | 61 | 25 | 0 | 0 | 0 | 0 | 742 |  |
| 15:15 | 15:30 | 0 | 2 | 47 | 0 | 0 | 3 | 0 | 53 | 0 | 0 | 63 | 15 | 0 | 0 | 0 | 0 | 770 |  |
| 15:30 | 15:45 | 0 | 6 | 32 | 0 | 0 | 3 | 0 | 77 | 0 | 0 | 66 | 23 | 0 | 0 | 0 | 0 | 764 |  |
| 15:45 | 16:00 | 0 | 1 | 42 | 0 | 0 | 0 | 1 | 60 | 0 | 0 | 59 | 20 | 0 | 0 | 0 | 0 | 773 | Peak |
| 16:00 | 16:15 | 0 | 2 | 32 | 0 | 0 | 1 | 0 | 69 | 0 | 0 | 62 | 31 | 0 | 0 | 0 | 0 | 760 |  |
| 16:15 | 16:30 | 0 | 1 | 27 | 0 | 0 | 2 | 0 | 68 | 0 | 0 | 57 | 22 | 0 | 0 | 0 | 0 | 736 |  |
| 16:30 | 16:45 | 0 | 1 | 37 | 0 | 0 | 1 | 0 | 74 | 0 | 0 | 79 | 24 | 0 | 0 | 0 | 0 | 758 |  |
| 16:45 | 17:00 | 0 | 0 | 36 | 0 | 0 | 2 | 1 | 56 | 0 | 0 | 53 | 22 | 0 | 0 | 0 | 0 | 680 |  |
| 17:00 | 17:15 | 0 | 0 | 32 | 0 | 0 | 0 | 0 | 53 | 0 | 0 | 64 | 24 | 0 | 0 | 0 | 0 | 645 |  |
| 17:15 | 17:30 | 0 | 1 | 40 | 0 | 0 | 5 | 0 | 73 | 0 | 0 | 51 | 29 | 0 | 0 | 0 | 0 |  |  |
| 17:30 | 17:45 | 0 | 0 | 30 | 0 | 0 | 0 | 0 | 46 | 0 | 0 | 41 | 21 | 0 | 0 | 0 | 0 |  |  |
| 17:45 | 18:00 | 0 | 0 | 20 | 0 | 0 | 1 | 0 | 62 | 0 | 0 | 30 | 22 | 0 | 0 | 0 | 0 |  |  |


| Peak Time |  | North Approach Belinda St |  |  |  | East Approach Off-ramp Princess Hw |  |  |  | South Approach Belinda St |  |  |  | Vest Approach On-ramp Princess Hw |  |  |  | Peak total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Period Start | Period End | U | R | SB | L | U | R | WB | L | $u$ | R | NB | L | U | R | EB | L |  |
| 7:30 | 8:30 | 0 | 3 | 68 | 0 | 0 | 3 | 0 | 154 | 0 | 0 | 224 | 136 | 0 | 0 | 0 | 0 | 588 |
| 15:45 | 16:45 | 0 | 5 | 138 | 0 | 0 | 4 | 1 | 271 | 0 | 0 | 257 | 97 | 0 | 0 | 0 | 0 | 773 |

Note: Site sketch is for illustrating traffic flows. Direction is indicative only, drawing is not to scale and not an exact streets configuration.



Belinda St


## ANNEXURE D: TRAFFIC SURVEY RESULTS

(SHEET 3 OF 3)

## TRANS TRAFFIC SURVEY

T. 1300828882 - F. 1300838883 - E. traffic@trafficsurvey.com.au - W. www.trafficsurvey.com.au

| AUTOMATIC COUNT SUMMARY |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Street Name: | Sims Rd |  | Location : |  intoreactinnc of Sime Rand |  |
| Suburb : | Gerringong |  | Start Date : | 00:00 Thu 11/February/2021 |  |
| Tubecounter ID | Y629DPQ5/P |  | Finish Date : | 00:00 Thu 18/February/2021 |  |
| Site ID Number : | 1523 |  | Speed Zone : | $70 \mathrm{~km} / \mathrm{h}$ |  |
| Prepared By : | Vo Son Binh |  | Email: | binh@trafficsurvey.com.au |  |
| GPS information | Lat $34^{\circ} 44^{\prime} 36.15$ South Long $150^{\circ} 49^{\prime} 0.85$ East |  | Direction of Travel |  |  |
|  |  |  | Both directions | Northbound | Southbound |
| Traffic Volume : |  | Weekdays Average | 166 | 83 | 83 |
| (Vehicles/Day) |  | 7 Day Average | 165 | 84 | 81 |
| Weekday | AM | 11:00 | 16 | 8 | 8 |
| Peak hour starts | PM | 15:00 | 20 | 10 | 10 |
| Speeds: |  | 85th Percentile | 66.6 | 65.4 | 67.9 |
| ( $\mathrm{Km} / \mathrm{Hr}$ ) |  | Average | 60.3 | 58.5 | 61.6 |
| Classification \% : |  | Light Vehicles up to 5.5m | 92.9\% | 92.9\% | 92.9\% |



QUALITY ASSURED COMPANY BY ISO 9001:2015
OH\&S SYSTEM CERTIFIED TO ISO 4801:2001
ENVIRONMENT MANAGEMENT SYSTEM CERTIFIED TO ISO14001:2015

## TRANS TRAFFIC SURVEY

Site
Direction Both directions $\quad$ Back to Site Summary Page

| Day | Monday | Tuesday | Wednesday | Thursday | Friday | Saturday | Sunday |  |  |  | day |  | end |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Date | 15/02/2021 | 16/02/2021 | 17/02/2021 | 11/02/2021 | 12/02/2021 | 13/02/2021 | 14/02/2021 | Total | Average | Total | Average | Total | Average |
| AM Peak | 07:00 | 11:00 | 11:00 | 08:00 | 10:00 | 10:00 | 10:00 | N/A | 11:00 | N/A | 11:00 | N/A | 10:00 |
| PM Peak | 15:00 | 15:00 | 15:00 | 15:00 | 15:00 | 17:00 | 14:00 | N/A | 15:00 | N/A | 15:00 | N/A | 14:00 |
| 00:00 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 1 |
| 01:00 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 3 | 0 | 2 | 0 | 1 | 1 |
| 02:00 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 2 | 1 |
| 03:00 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 |
| 04:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 05:00 | 0 | 0 | 3 | 1 | 2 | 0 | 0 | 6 | 1 | 6 | 1 | 0 | 0 |
| 06:00 | 5 | 2 | 3 | 3 | 2 | 0 | 3 | 18 | 3 | 15 | 3 | 3 | 2 |
| 07:00 | 10 | 5 | 11 | 8 | 5 | 3 | 2 | 44 | 6 | 39 | 8 | 5 | 3 |
| 08:00 | 10 | 11 | 12 | 17 | 14 | 13 | 6 | 83 | 12 | 64 | 13 | 19 | 10 |
| 09:00 | 7 | 11 | 12 | 13 | 16 | 10 | 14 | 83 | 12 | 59 | 12 | 24 | 12 |
| 10:00 | 4 | 16 | 9 | 11 | 18 | 18 | 16 | 92 | 13 | 58 | 12 | 34 | 17 |
| 11:00 | 10 | 19 | 18 | 16 | 16 | 13 | 10 | 102 | 15 | 79 | 16 | 23 | 12 |
| 12:00 | 18 | 9 | 12 | 12 | 10 | 9 | 23 | 93 | 13 | 61 | 12 | 32 | 16 |
| 13:00 | 14 | 8 | 11 | 16 | 19 | 14 | 15 | 97 | 14 | 68 | 14 | 29 | 15 |
| 14:00 | 8 | 18 | 12 | 14 | 9 | 15 | 28 | 104 | 15 | 61 | 12 | 43 | 22 |
| 15:00 | 22 | 22 | 14 | 23 | 20 | 3 | 23 | 127 | 18 | 101 | 20 | 26 | 13 |
| 16:00 | 15 | 6 | 13 | 10 | 17 | 8 | 13 | 82 | 12 | 61 | 12 | 21 | 11 |
| 17:00 | 9 | 9 | 9 | 5 | 8 | 19 | 10 | 69 | 10 | 40 | 8 | 29 | 15 |
| 18:00 | 8 | 11 | 3 | 10 | 9 | 15 | 11 | 67 | 10 | 41 | 8 | 26 | 13 |
| 19:00 | 5 | 5 | 2 | 12 | 0 | 9 | 4 | 37 | 5 | 24 | 5 | 13 | 7 |
| 20:00 | 3 | 4 | 2 | 8 | 8 | 5 | 5 | 35 | 5 | 25 | 5 | 10 | 5 |
| 21:00 | 3 | 1 | 2 | 2 | 1 | 1 | 0 | 10 | 1 | 9 | 2 | 1 | 1 |
| 22:00 | 2 | 6 | 0 | 0 | 3 | 5 | 2 | 18 | 3 | 11 | 2 | 7 | 4 |
| 23:00 | 2 | 0 | 1 | 0 | 4 | 0 | 0 | 7 | 1 | 7 | 1 | 0 | 0 |
| Total | 155 | 166 | 149 | 181 | 182 | 163 | 186 | 1182 | 169 | 833 9.36\% 166 |  | 349 | 181 |
| \% Heavy | 6.45\% | 12.05\% | 10.74\% | 9.94\% | 7.69\% | 1.84\% | 1.61\% | 7.11\% |  | 9.36\% |  | 1.72\% |  |



ANNEXURE E: SIDRA INTERSECTION RESULTS (12 SHEETS)

## MOVEMENT SUMMARY

$\nabla$ Site: 101 [(EX AM) Princes / Sims / Belinda North (Site Folder:
General)]
Sims Road / Belinda Street (North) / Princes Highway (Northbound)
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | UT MES HV ] veh/h |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay $\qquad$ | Level of Service | $\begin{gathered} \text { 95\% B B } \\ \text { QU } \\ \text { [ Veh. } \\ \text { veh } \end{gathered}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \mathrm{m} \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 16 | 2 | 16 | 12.5 | 0.143 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.56 | 0.00 | 50.7 |
| 2 T1 | 15 | 3 | 15 | 20.0 | 0.143 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.56 | 0.00 | 53.9 |
| 3 R 2 | 223 | 12 | 223 | 5.4 | 0.143 | 4.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.56 | 0.00 | 50.0 |
| Approach | 254 | 17 | 254 | 6.7 | 0.143 | 3.7 | NA | 0.0 | 0.0 | 0.00 | 0.56 | 0.00 | 50.3 |
| North: Sims Road (N) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 5 | 1 | 5 | 20.0 | 0.012 | 6.7 | LOS A | 0.0 | 0.4 | 0.30 | 0.53 | 0.30 | 53.1 |
| 8 T1 | 7 | 2 | 7 | 28.6 | 0.012 | 5.4 | LOSA | 0.0 | 0.4 | 0.30 | 0.53 | 0.30 | 48.2 |
| 9 R2 | 1 | 0 | 1 | 0.0 | 0.012 | 5.9 | LOS A | 0.0 | 0.4 | 0.30 | 0.53 | 0.30 | 52.7 |
| Approach | 13 | 3 | 13 | 23.1 | 0.012 | 5.9 | LOS A | 0.0 | 0.4 | 0.30 | 0.53 | 0.30 | 51.0 |
| West: Princes Highway (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 1 | 0 | 1 | 0.0 | 0.111 | 8.1 | LOS A | 0.4 | 2.9 | 0.32 | 0.93 | 0.32 | 51.4 |
| 11 T1 | 7 | 1 | 7 | 14.3 | 0.111 | 9.7 | LOS A | 0.4 | 2.9 | 0.32 | 0.93 | 0.32 | 50.6 |
| 12 R 2 | 83 | 8 | 83 | 9.6 | 0.111 | 9.4 | LOS A | 0.4 | 2.9 | 0.32 | 0.93 | 0.32 | 44.9 |
| Approach | 91 | 9 | 91 | 9.9 | 0.111 | 9.4 | LOS A | 0.4 | 2.9 | 0.32 | 0.93 | 0.32 | 45.7 |
| All <br> Vehicles | 358 | 29 | 358 | 8.1 | 0.143 | 5.3 | NA | 0.4 | 2.9 | 0.09 | 0.66 | 0.09 | 49.0 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: \1192.168.1.107\mte storage\Jobs\2020\200908IMTE SIDRAI21 0215 - Existing.sip9

## MOVEMENT SUMMARY

$\nabla$ Site: 101 [(EX PM) Princes / Sims / Belinda North (Site Folder:
General)]
Sims Road / Belinda Street (North) / Princes Highway (Northbound)
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | MES <br> HV ] veh/h | $\begin{aligned} & \text { DEN } \\ & \text { FL } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay sec $\qquad$ | Level of Service | $\begin{aligned} & \text { 95\% B B } \\ & \text { QU } \\ & \text { [ Veh. } \\ & \text { veh } \end{aligned}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \mathrm{m} \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| L2 | 30 | 14 | 30 | 46.7 | 0.160 | 3.9 | LOSA | 0.0 | 0.0 | 0.00 | 0.54 | 0.00 | 48.4 |
| 2 T1 | 27 | 12 | 27 | 44.4 | 0.160 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.54 | 0.00 | 53.9 |
| 3 R2 | 223 | 2 | 223 | 0.9 | 0.160 | 4.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.54 | 0.00 | 50.4 |
| Approach | 280 | 28 | 280 | 10.0 | 0.160 | 3.6 | NA | 0.0 | 0.0 | 0.00 | 0.54 | 0.00 | 50.5 |
| North: Sims Road (N) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 4 | 1 | 4 | 25.0 | 0.012 | 6.8 | LOSA | 0.0 | 0.4 | 0.31 | 0.53 | 0.31 | 53.0 |
| 8 T1 | 8 | 2 | 8 | 25.0 | 0.012 | 5.3 | LOSA | 0.0 | 0.4 | 0.31 | 0.53 | 0.31 | 48.4 |
| 9 R2 | 1 | 0 | 1 | 0.0 | 0.012 | 6.3 | LOSA | 0.0 | 0.4 | 0.31 | 0.53 | 0.31 | 52.8 |
| Approach | 13 | 3 | 13 | 23.1 | 0.012 | 5.8 | LOSA | 0.0 | 0.4 | 0.31 | 0.53 | 0.31 | 50.7 |
| West: Princes Highway (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 1 | 0 | 1 | 0.0 | 0.204 | 8.2 | LOS A | 0.8 | 5.5 | 0.39 | 0.93 | 0.39 | 51.3 |
| 11 T1 | 9 | 0 | 9 | 0.0 | 0.204 | 9.0 | LOS A | 0.8 | 5.5 | 0.39 | 0.93 | 0.39 | 51.0 |
| 12 R 2 | 156 | 9 | 156 | 5.8 | 0.204 | 9.6 | LOSA | 0.8 | 5.5 | 0.39 | 0.93 | 0.39 | 44.8 |
| Approach | 166 | 9 | 166 | 5.4 | 0.204 | 9.5 | LOS A | 0.8 | 5.5 | 0.39 | 0.93 | 0.39 | 45.4 |
| All <br> Vehicles | 459 | 40 | 459 | 8.7 | 0.204 | 5.8 | NA | 0.8 | 5.5 | 0.15 | 0.68 | 0.15 | 48.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: \1192.168.1.107\mte storage\Jobs\2020\200908IMTE SIDRAI21 0215 - Existing.sip9

## MOVEMENT SUMMARY

Site: 101 [(EX AM) Princes / Belinda South (Site Folder:
General)]
Belinda Street / Princes Highway (Southbound)
Site Category: (None)
Stop (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | JT MES HV] veh/h |  | $\begin{aligned} & \text { ND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay $\qquad$ | Level of Service | $\begin{aligned} & 95 \% \text { B } \\ & \text { QU } \\ & \text { [ Veh. } \\ & \text { veh } \end{aligned}$ | CK OF UE Dist ] m | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed $\mathrm{km} / \mathrm{h}$ |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 136 | 11 | 136 | 8.1 | 0.197 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.0 |
| 2 T1 | 224 | 13 | 224 | 5.8 | 0.197 | 0.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.5 |
| Approach | 360 | 24 | 360 | 6.7 | 0.197 | 2.2 | NA | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.2 |
| East: Princes Highway (E) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 154 | 18 | 154 | 11.7 | 0.128 | 8.8 | LOS A | 0.5 | 4.2 | 0.18 | 0.91 | 0.18 | 51.3 |
| 5 T1 | 1 | 0 | 1 | 0.0 | 0.128 | 10.7 | LOS B | 0.5 | 4.2 | 0.18 | 0.91 | 0.18 | 51.5 |
| 6 R2 | 3 | 1 | 3 | 33.3 | 0.128 | 11.8 | LOS B | 0.5 | 4.2 | 0.18 | 0.91 | 0.18 | 35.2 |
| Approach | 158 | 19 | 158 | 12.0 | 0.128 | 8.9 | LOS A | 0.5 | 4.2 | 0.18 | 0.91 | 0.18 | 51.0 |
| North: Belinda Street ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 68 | 8 | 68 | 11.8 | 0.042 | 0.2 | LOS A | 0.0 | 0.4 | 0.06 | 0.02 | 0.06 | 59.3 |
| 9 R2 | 3 | 2 | 3 | 66.7 | 0.042 | 6.4 | LOS A | 0.0 | 0.4 | 0.06 | 0.02 | 0.06 | 49.6 |
| Approach | 71 | 10 | 71 | 14.1 | 0.042 | 0.5 | NA | 0.0 | 0.4 | 0.06 | 0.02 | 0.06 | 58.9 |
| All <br> Vehicles | 589 | 53 | 589 | 9.0 | 0.197 | 3.8 | NA | 0.5 | 4.2 | 0.06 | 0.38 | 0.06 | 54.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [(EX PM) Princes / Belinda South (Site Folder:
General)]
Belinda Street / Princes Highway (Southbound)
Site Category: (None)
Stop (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | Tis <br> HV ] veh/h |  | $\begin{aligned} & \text { ND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay sec | Level of Service | $\begin{gathered} 95 \% \text { B } \\ \text { QU } \\ \text { [ Veh. } \\ \text { veh } \end{gathered}$ | OF JE Dist $]$ m | $\begin{aligned} & \text { Prop. } \\ & \text { Out } \end{aligned}$ | Effective Stop Rate |  | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| L2 | 97 | 6 | 97 | 6.2 | 0.190 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 56.6 |
| 2 T1 | 257 | 12 | 257 | 4.7 | 0.190 | 0.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 57.4 |
| Approach | 354 | 18 | 354 | 5.1 | 0.190 | 1.6 | NA | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 57.1 |
| East: Princes Highway (E) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 271 | 13 | 271 | 4.8 | 0.238 | 8.9 | LOS A | 1.1 | 8.0 | 0.30 | 0.88 | 0.30 | 51.4 |
| 5 T1 | 5 | 1 | 5 | 20.0 | 0.238 | 13.7 | LOS B | 1.1 | 8.0 | 0.30 | 0.88 | 0.30 | 50.5 |
| 6 R2 | 4 | 1 | 4 | 25.0 | 0.238 | 13.2 | LOS B | 1.1 | 8.0 | 0.30 | 0.88 | 0.30 | 35.2 |
| Approach | 280 | 15 | 280 | 5.4 | 0.238 | 9.1 | LOSA | 1.1 | 8.0 | 0.30 | 0.88 | 0.30 | 51.2 |
| North: Belinda Street ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 138 | 10 | 138 | 7.2 | 0.079 | 0.1 | LOS A | 0.1 | 0.4 | 0.04 | 0.02 | 0.04 | 59.5 |
| 9 R2 | 5 | 1 | 5 | 20.0 | 0.079 | 5.6 | LOSA | 0.1 | 0.4 | 0.04 | 0.02 | 0.04 | 53.5 |
| Approach | 143 | 11 | 143 | 7.7 | 0.079 | 0.3 | NA | 0.1 | 0.4 | 0.04 | 0.02 | 0.04 | 59.3 |
| All Vehicles | 777 | 44 | 777 | 5.7 | 0.238 | 4.0 | NA | 1.1 | 8.0 | 0.12 | 0.40 | 0.12 | 54.4 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 101 [(FUT AM - AVG) Princes / Sims / Belinda North (Site
Folder: General)]
Sims Road / Belinda Street (North) / Princes Highway (Northbound)
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | UT MES HV ] veh/h |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay sec $\qquad$ | Level of Service | $\begin{aligned} & \text { 95\% B B } \\ & \text { QU } \\ & \text { [ Veh. } \\ & \text { veh } \end{aligned}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \mathrm{m} \end{gathered}$ | Prop. Que | Effective Stop Rate | $\begin{aligned} & \text { Aver. } \\ & \text { No. } \\ & \text { Cycles } \end{aligned}$ | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 16 | 2 | 17 | 12.5 | 0.161 | 3.9 | LOSA | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 51.0 |
| 2 T1 | 34 | 3 | 36 | 8.8 | 0.161 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 54.3 |
| 3 R2 | 223 | 12 | 235 | 5.4 | 0.161 | 4.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 50.4 |
| Approach | 273 | 17 | 287 | 6.2 | 0.161 | 3.5 | NA | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 50.9 |
| North: Sims Road (N) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 11 | 1 | 12 | 9.1 | 0.022 | 6.5 | LOSA | 0.1 | 0.6 | 0.32 | 0.55 | 0.32 | 53.4 |
| 8 T1 | 12 | 2 | 13 | 16.7 | 0.022 | 5.2 | LOSA | 0.1 | 0.6 | 0.32 | 0.55 | 0.32 | 48.3 |
| 9 R2 | 1 | 0 | 1 | 0.0 | 0.022 | 6.0 | LOSA | 0.1 | 0.6 | 0.32 | 0.55 | 0.32 | 52.6 |
| Approach | 24 | 3 | 25 | 12.5 | 0.022 | 5.9 | LOSA | 0.1 | 0.6 | 0.32 | 0.55 | 0.32 | 51.4 |
| West: Princes Highway (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 17 | 0 | 18 | 0.0 | 0.135 | 8.2 | LOS A | 0.5 | 3.6 | 0.23 | 0.95 | 0.23 | 51.2 |
| 11 T1 | 7 | 1 | 7 | 14.3 | 0.135 | 9.9 | LOS A | 0.5 | 3.6 | 0.23 | 0.95 | 0.23 | 50.4 |
| 12 R 2 | 83 | 8 | 87 | 9.6 | 0.135 | 9.8 | LOSA | 0.5 | 3.6 | 0.23 | 0.95 | 0.23 | 44.6 |
| Approach | 107 | 9 | 113 | 8.4 | 0.135 | 9.5 | LOS A | 0.5 | 3.6 | 0.23 | 0.95 | 0.23 | 46.6 |
| All <br> Vehicles | 404 | 29 | 425 | 7.2 | 0.161 | 5.2 | NA | 0.5 | 3.6 | 0.08 | 0.64 | 0.08 | 49.6 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

$\nabla$ Site: 101 [(FUT PM - AVG) Princes / Sims / Belinda North (Site
Folder: General)]
Sims Road / Belinda Street (North) / Princes Highway (Northbound)
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { INF } \\ & \text { VOL } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | UT MES HV] veh/h |  | $\begin{aligned} & \text { ND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay sec $\qquad$ | Level of Service | $\begin{aligned} & \text { 95\% B B } \\ & \text { QU } \\ & \text { [ Veh. } \\ & \text { veh } \end{aligned}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \text { m } \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> $\mathrm{km} / \mathrm{h}$ |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 16 | 2 | 17 | 12.5 | 0.153 | 3.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.56 | 0.00 | 50.7 |
| 2 T 1 | 17 | 3 | 18 | 17.6 | 0.153 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.56 | 0.00 | 53.9 |
| 3 R2 | 229 | 8 | 241 | 3.5 | 0.153 | 4.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.56 | 0.00 | 50.2 |
| Approach | 262 | 13 | 276 | 5.0 | 0.153 | 3.7 | NA | 0.0 | 0.0 | 0.00 | 0.56 | 0.00 | 50.4 |
| North: Sims Road (N) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 22 | 1 | 23 | 4.5 | 0.047 | 6.5 | LOS A | 0.2 | 1.3 | 0.32 | 0.56 | 0.32 | 53.7 |
| 8 T1 | 30 | 2 | 32 | 6.7 | 0.047 | 5.1 | LOS A | 0.2 | 1.3 | 0.32 | 0.56 | 0.32 | 48.5 |
| 9 R2 | 1 | 0 | 1 | 0.0 | 0.047 | 6.3 | LOSA | 0.2 | 1.3 | 0.32 | 0.56 | 0.32 | 52.7 |
| Approach | 53 | 3 | 56 | 5.7 | 0.047 | 5.7 | LOS A | 0.2 | 1.3 | 0.32 | 0.56 | 0.32 | 51.4 |
| West: Princes Highway (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 5 | 0 | 5 | 0.0 | 0.221 | 8.1 | LOSA | 0.8 | 6.0 | 0.32 | 0.95 | 0.32 | 51.1 |
| 11 T1 | 9 | 0 | 9 | 0.0 | 0.221 | 9.0 | LOSA | 0.8 | 6.0 | 0.32 | 0.95 | 0.32 | 50.9 |
| 12 R 2 | 155 | 8 | 163 | 5.2 | 0.221 | 9.7 | LOSA | 0.8 | 6.0 | 0.32 | 0.95 | 0.32 | 44.6 |
| Approach | 169 | 8 | 178 | 4.7 | 0.221 | 9.6 | LOSA | 0.8 | 6.0 | 0.32 | 0.95 | 0.32 | 45.4 |
| All <br> Vehicles | 484 | 24 | 509 | 5.0 | 0.221 | 6.0 | NA | 0.8 | 6.0 | 0.15 | 0.70 | 0.15 | 48.7 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

Site: 101 [(FUT AM - AVG) Princes / Belinda South (Site Folder: General)]
Belinda Street / Princes Highway (Southbound)
Site Category: (None)
Stop (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | JT MES HV ] veh/h |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | $\begin{gathered} 95 \% \text { B } \\ \text { QU } \\ \text { [ Veh. } \\ \text { veh } \end{gathered}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \text { m } \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver No. Cycles | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 136 | 11 | 143 | 8.1 | 0.209 | 5.7 | LOSA | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.1 |
| 2 T1 | 228 | 13 | 240 | 5.7 | 0.209 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.5 |
| Approach | 364 | 24 | 383 | 6.6 | 0.209 | 2.2 | NA | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.3 |
| East: Princes Highway (E) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 154 | 18 | 162 | 11.7 | 0.160 | 8.9 | LOS A | 0.7 | 5.2 | 0.19 | 0.92 | 0.19 | 51.2 |
| 5 T1 | 1 | 0 | 1 | 0.0 | 0.160 | 11.1 | LOS B | 0.7 | 5.2 | 0.19 | 0.92 | 0.19 | 51.4 |
| 6 R2 | 19 | 1 | 20 | 5.3 | 0.160 | 10.4 | LOS B | 0.7 | 5.2 | 0.19 | 0.92 | 0.19 | 35.5 |
| Approach | 174 | 19 | 183 | 10.9 | 0.160 | 9.0 | LOSA | 0.7 | 5.2 | 0.19 | 0.92 | 0.19 | 49.8 |
| North: Belinda Street ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 69 | 8 | 73 | 11.6 | 0.049 | 0.3 | LOS A | 0.1 | 0.7 | 0.13 | 0.06 | 0.13 | 58.5 |
| 9 R2 | 8 | 2 | 8 | 25.0 | 0.049 | 5.8 | LOSA | 0.1 | 0.7 | 0.13 | 0.06 | 0.13 | 52.3 |
| Approach | 77 | 10 | 81 | 13.0 | 0.049 | 0.9 | NA | 0.1 | 0.7 | 0.13 | 0.06 | 0.13 | 57.8 |
| All <br> Vehicles | 615 | 53 | 647 | 8.6 | 0.209 | 4.0 | NA | 0.7 | 5.2 | 0.07 | 0.40 | 0.07 | 53.9 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

Site: 101 [(FUT PM - AVG) Princes / Belinda South (Site Folder: General)]
Belinda Street / Princes Highway (Southbound)
Site Category: (None)
Stop (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | $\begin{aligned} & \text { INP } \\ & \text { VOLu } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | JT MES HV $]$ veh/h |  | $\begin{aligned} & \text { WD } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service |  | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \text { m } \end{gathered}$ | Prop. Que | Effective Stop Rate | $\begin{aligned} & \text { Aver. } \\ & \text { No. } \\ & \text { Cycles } \end{aligned}$ | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 97 | 6 | 102 | 6.2 | 0.201 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 56.6 |
| 2 T1 | 258 | 12 | 272 | 4.7 | 0.201 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 57.4 |
| Approach | 355 | 18 | 374 | 5.1 | 0.201 | 1.6 | NA | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 57.1 |
| East: Princes Highway (E) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 271 | 13 | 285 | 4.8 | 0.264 | 9.0 | LOS A | 1.2 | 9.0 | 0.32 | 0.89 | 0.32 | 51.3 |
| 5 T1 | 5 | 1 | 5 | 20.0 | 0.264 | 14.6 | LOS B | 1.2 | 9.0 | 0.32 | 0.89 | 0.32 | 50.5 |
| 6 R2 | 9 | 1 | 9 | 11.1 | 0.264 | 12.8 | LOS B | 1.2 | 9.0 | 0.32 | 0.89 | 0.32 | 35.3 |
| Approach | 285 | 15 | 300 | 5.3 | 0.264 | 9.2 | LOSA | 1.2 | 9.0 | 0.32 | 0.89 | 0.32 | 50.9 |
| North: Belinda Street ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 142 | 10 | 149 | 7.0 | 0.101 | 0.4 | LOS A | 0.2 | 1.6 | 0.16 | 0.09 | 0.16 | 58.1 |
| 9 R2 | 23 | 1 | 24 | 4.3 | 0.101 | 5.5 | LOS A | 0.2 | 1.6 | 0.16 | 0.09 | 0.16 | 53.7 |
| Approach | 165 | 11 | 174 | 6.7 | 0.101 | 1.1 | NA | 0.2 | 1.6 | 0.16 | 0.09 | 0.16 | 57.5 |
| All <br> Vehicles | 805 | 44 | 847 | 5.5 | 0.264 | 4.2 | NA | 1.2 | 9.0 | 0.14 | 0.40 | 0.14 | 54.1 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

$\nabla$ Site: 101 [(FUT AM - HIGH) Princes / Sims / Belinda North
(Site Folder: General)]
Sims Road / Belinda Street (North) / Princes Highway (Northbound)
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | UT MES HV ] veh/h |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay sec $\qquad$ | Level of Service | $\begin{aligned} & \text { 95\% B B } \\ & \text { QU } \\ & \text { [ Veh. } \\ & \text { veh } \end{aligned}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \mathrm{m} \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 16 | 2 | 17 | 12.5 | 0.168 | 3.9 | LOSA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 51.3 |
| 2 T1 | 48 | 3 | 51 | 6.3 | 0.168 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 54.6 |
| 3 R2 | 223 | 12 | 235 | 5.4 | 0.168 | 4.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 50.6 |
| Approach | 287 | 17 | 302 | 5.9 | 0.168 | 3.3 | NA | 0.0 | 0.0 | 0.00 | 0.51 | 0.00 | 51.2 |
| North: Sims Road (N) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 13 | 1 | 14 | 7.7 | 0.026 | 6.5 | LOSA | 0.1 | 0.7 | 0.32 | 0.55 | 0.32 | 53.5 |
| 8 T1 | 15 | 2 | 16 | 13.3 | 0.026 | 5.2 | LOSA | 0.1 | 0.7 | 0.32 | 0.55 | 0.32 | 48.3 |
| 9 R2 | 1 | 0 | 1 | 0.0 | 0.026 | 6.1 | LOSA | 0.1 | 0.7 | 0.32 | 0.55 | 0.32 | 52.6 |
| Approach | 29 | 3 | 31 | 10.3 | 0.026 | 5.8 | LOS A | 0.1 | 0.7 | 0.32 | 0.55 | 0.32 | 51.4 |
| West: Princes Highway (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 28 | 0 | 29 | 0.0 | 0.146 | 8.2 | LOS A | 0.5 | 4.0 | 0.23 | 0.95 | 0.23 | 51.2 |
| 11 T1 | 7 | 1 | 7 | 14.3 | 0.146 | 10.0 | LOS B | 0.5 | 4.0 | 0.23 | 0.95 | 0.23 | 50.3 |
| 12 R2 | 83 | 8 | 87 | 9.6 | 0.146 | 10.0 | LOSA | 0.5 | 4.0 | 0.23 | 0.95 | 0.23 | 44.6 |
| Approach | 118 | 9 | 124 | 7.6 | 0.146 | 9.6 | LOS A | 0.5 | 4.0 | 0.23 | 0.95 | 0.23 | 47.1 |
| All <br> Vehicles | 434 | 29 | 457 | 6.7 | 0.168 | 5.2 | NA | 0.5 | 4.0 | 0.09 | 0.63 | 0.09 | 49.9 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

$\nabla$ Site: 101 [(FUT PM - HIGH) Princes / Sims / Belinda North
(Site Folder: General)]
Sims Road / Belinda Street (North) / Princes Highway (Northbound)
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | UT MES HV ] veh/h |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay $\qquad$ | Level of Service | $\begin{aligned} & 95 \% \text { B/ } \\ & \text { QUt } \\ & \text { [ Veh. } \\ & \text { veh } \end{aligned}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \mathrm{m} \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 16 | 2 | 17 | 12.5 | 0.155 | 3.9 | LOSA | 0.0 | 0.0 | 0.00 | 0.55 | 0.00 | 50.8 |
| 2 T 1 | 21 | 3 | 22 | 14.3 | 0.155 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.55 | 0.00 | 54.0 |
| 3 R2 | 229 | 8 | 241 | 3.5 | 0.155 | 4.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.55 | 0.00 | 50.2 |
| Approach | 266 | 13 | 280 | 4.9 | 0.155 | 3.7 | NA | 0.0 | 0.0 | 0.00 | 0.55 | 0.00 | 50.6 |
| North: Sims Road (N) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 33 | 1 | 35 | 3.0 | 0.068 | 6.5 | LOSA | 0.3 | 1.8 | 0.32 | 0.57 | 0.32 | 53.7 |
| 8 T1 | 44 | 2 | 46 | 4.5 | 0.068 | 5.1 | LOSA | 0.3 | 1.8 | 0.32 | 0.57 | 0.32 | 48.5 |
| 9 R2 | 1 | 0 | 1 | 0.0 | 0.068 | 6.3 | LOSA | 0.3 | 1.8 | 0.32 | 0.57 | 0.32 | 52.7 |
| Approach | 78 | 3 | 82 | 3.8 | 0.068 | 5.7 | LOSA | 0.3 | 1.8 | 0.32 | 0.57 | 0.32 | 51.4 |
| West: Princes Highway (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 8 | 0 | 8 | 0.0 | 0.228 | 8.1 | LOS A | 0.9 | 6.2 | 0.31 | 0.96 | 0.31 | 51.0 |
| 11 T1 | 9 | 0 | 9 | 0.0 | 0.228 | 9.1 | LOSA | 0.9 | 6.2 | 0.31 | 0.96 | 0.31 | 50.8 |
| 12 R 2 | 155 | 8 | 163 | 5.2 | 0.228 | 9.9 | LOSA | 0.9 | 6.2 | 0.31 | 0.96 | 0.31 | 44.5 |
| Approach | 172 | 8 | 181 | 4.7 | 0.228 | 9.8 | LOSA | 0.9 | 6.2 | 0.31 | 0.96 | 0.31 | 45.4 |
| All <br> Vehicles | 516 | 24 | 543 | 4.7 | 0.228 | 6.0 | NA | 0.9 | 6.2 | 0.15 | 0.69 | 0.15 | 48.9 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

Site: 101 [(FUT AM - HIGH) Princes / Belinda South (Site Folder: General)]
Belinda Street / Princes Highway (Southbound)
Site Category: (None)
Stop (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | JT MES HV] veh/h |  | $\begin{aligned} & \text { ND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay $\qquad$ | Level of Service | $\begin{aligned} & 95 \% \text { B } \\ & \text { QU } \\ & \text { [ Veh. } \\ & \text { veh } \end{aligned}$ | CK OF UE Dist ] m | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. <br> Speed <br> $\mathrm{km} / \mathrm{h}$ |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 136 | 11 | 143 | 8.1 | 0.210 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.1 |
| 2 T1 | 230 | 13 | 242 | 5.7 | 0.210 | 0.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.5 |
| Approach | 366 | 24 | 385 | 6.6 | 0.210 | 2.1 | NA | 0.0 | 0.0 | 0.00 | 0.22 | 0.00 | 56.3 |
| East: Princes Highway (E) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 154 | 18 | 162 | 11.7 | 0.177 | 8.9 | LOS A | 0.8 | 5.7 | 0.20 | 0.92 | 0.20 | 51.1 |
| 5 T1 | 1 | 0 | 1 | 0.0 | 0.177 | 11.2 | LOS B | 0.8 | 5.7 | 0.20 | 0.92 | 0.20 | 51.3 |
| 6 R2 | 30 | 1 | 32 | 3.3 | 0.177 | 10.3 | LOS B | 0.8 | 5.7 | 0.20 | 0.92 | 0.20 | 35.5 |
| Approach | 185 | 19 | 195 | 10.3 | 0.177 | 9.1 | LOS A | 0.8 | 5.7 | 0.20 | 0.92 | 0.20 | 49.0 |
| North: Belinda Street ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 70 | 8 | 74 | 11.4 | 0.051 | 0.4 | LOS A | 0.1 | 0.8 | 0.15 | 0.08 | 0.15 | 58.2 |
| 9 R2 | 10 | 2 | 11 | 20.0 | 0.051 | 5.7 | LOS A | 0.1 | 0.8 | 0.15 | 0.08 | 0.15 | 52.5 |
| Approach | 80 | 10 | 84 | 12.5 | 0.051 | 1.1 | NA | 0.1 | 0.8 | 0.15 | 0.08 | 0.15 | 57.5 |
| All <br> Vehicles | 631 | 53 | 664 | 8.4 | 0.210 | 4.1 | NA | 0.8 | 5.7 | 0.08 | 0.41 | 0.08 | 53.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [(FUT PM - HIGH) Princes / Belinda South (Site Folder: General)]
Belinda Street / Princes Highway (Southbound)
Site Category: (None)
Stop (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | $\begin{aligned} & \text { INP } \\ & \text { VOLu } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | JT MES HV $]$ veh/h |  | $\begin{aligned} & \text { WD } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service |  | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \text { m } \end{gathered}$ | Prop. Que | Effective Stop Rate | $\begin{aligned} & \text { Aver. } \\ & \text { No. } \\ & \text { Cycles } \end{aligned}$ | Aver. Speed <br> km/h |
| South: Belinda Street (S) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 97 | 6 | 102 | 6.2 | 0.201 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 56.6 |
| 2 T1 | 259 | 12 | 273 | 4.6 | 0.201 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 57.4 |
| Approach | 356 | 18 | 375 | 5.1 | 0.201 | 1.6 | NA | 0.0 | 0.0 | 0.00 | 0.16 | 0.00 | 57.1 |
| East: Princes Highway (E) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 271 | 13 | 285 | 4.8 | 0.270 | 9.0 | LOS A | 1.3 | 9.2 | 0.32 | 0.89 | 0.32 | 51.3 |
| 5 T1 | 5 | 1 | 5 | 20.0 | 0.270 | 14.9 | LOS B | 1.3 | 9.2 | 0.32 | 0.89 | 0.32 | 50.4 |
| 6 R2 | 12 | 1 | 13 | 8.3 | 0.270 | 12.8 | LOS B | 1.3 | 9.2 | 0.32 | 0.89 | 0.32 | 35.3 |
| Approach | 288 | 15 | 303 | 5.2 | 0.270 | 9.3 | LOS A | 1.3 | 9.2 | 0.32 | 0.89 | 0.32 | 50.7 |
| North: Belinda Street ( N ) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 145 | 10 | 153 | 6.9 | 0.112 | 0.5 | LOS A | 0.3 | 2.3 | 0.21 | 0.13 | 0.21 | 57.5 |
| 9 R2 | 34 | 1 | 36 | 2.9 | 0.112 | 5.5 | LOS A | 0.3 | 2.3 | 0.21 | 0.13 | 0.21 | 53.3 |
| Approach | 179 | 11 | 188 | 6.1 | 0.112 | 1.4 | NA | 0.3 | 2.3 | 0.21 | 0.13 | 0.21 | 56.6 |
| All <br> Vehicles | 823 | 44 | 866 | 5.3 | 0.270 | 4.2 | NA | 1.3 | 9.2 | 0.16 | 0.41 | 0.16 | 53.9 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 9.0 | Copyright © 2000-2020 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: MCLAREN TRAFFIC ENGINEERING | Licence: PLUS / 1PC | Processed: Friday, 19 February 2021 3:56:18 PM
Project: \1192.168.1.107 Imte storage\Jobs\2020\200908\MTE SIDRAI21 0219 - Future.sip9

ANNEXURE F: SWEPT PATH ACCESS ANALYSIS
(SHEET 1 OF 3)


AUSTRALIAN STANDARD ARTICULATED VEHICLE (AV)


Truck \& Dog (H.YUNCKEN)
First Unit Width
Trailer Width
First Unit Track
Trailer Track
mm
$: 2500$
: 2500 2500 2500 2500

Lock to Lock Time Steering Angle Articulating Angle
6.0
39.0 6.0
$: 39.0$
$: 70.0$

## 19M TRUCK AND DOG VEHICLE

Blue - Tyre Path
Green - Vehicle Body
Red - 500mm Clearance

## ANNEXURE F: SWEPT PATH ACCESS ANALYSIS

(SHEET 2 OF 3)


20m AV - SOUTHBOUND AND NORTHBOUND ENTRY \& EXIT

## ANNEXURE F: SWEPT PATH ACCESS ANALYSIS

(SHEET 3 OF 3)


19m TRUCK AND DOG - SOUTHBOUND AND NORTHBOUND ENTRY \& EXIT

